

Cost-Benefit Analysis of High-Strength Materials for Safety-Related Nuclear Reinforced Concrete Shear Walls



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DOE-NE NEET-1 Program Goals

- Nuclear Energy Enabling Technologies Program-Advanced Methods for Manufacturing (NEET-1)
- “Accelerate innovations that reduce the cost and schedule of constructing new nuclear plants and make fabrication of nuclear power plant components faster, cheaper, and more reliable.”
- “Develop new/ revised nuclear industry codes and standards that enable the utilization of newly developed technologies.”

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Project Objective

Reduce field construction times and fabrication costs of reinforced concrete nuclear structures through:

- 1) High-strength rebar
- 2) Prefabricated rebar assemblies, including headed anchorages
- 3) High-strength concrete

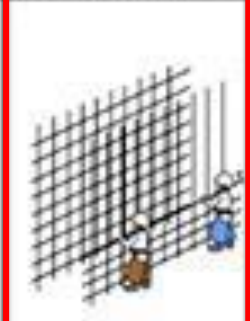

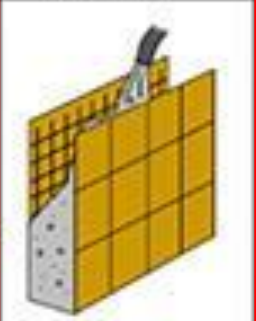
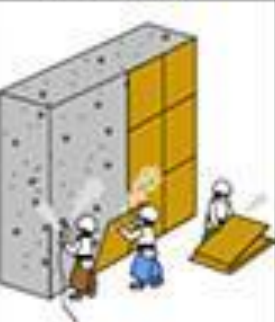
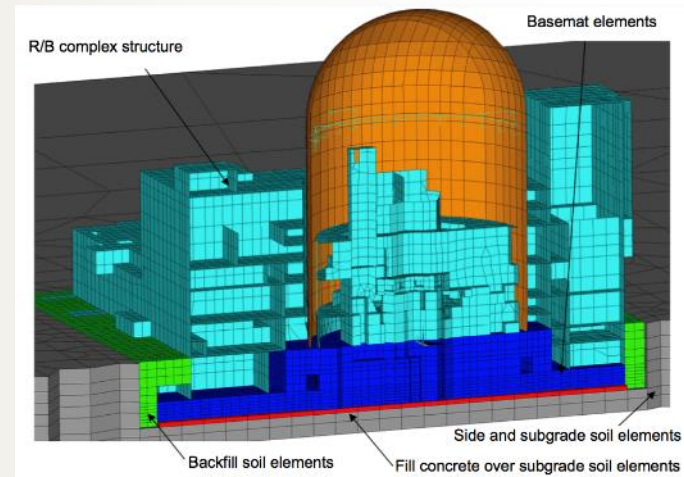
| Work Structure | Rebar arrangement | Form work (assembling) | Placing concrete | Form work (removal) |
|----------------|--|---|--|--|
| RC |  |  <i>Wooden form</i> |  |  |
| 28days | 13days | 7days | 4days | 4days |

Figure From:
MPR-2610 Rev 2
Sept. 2004



Project Scope

- Explore effectiveness, code conformity, and viability of existing high-strength materials
- Focus on stocky shear walls – predominant load resisting members in nuclear structures (pressure vessels not in scope)
- Aim to reduce complexities in rebar to improve construction quality and ease of inspection

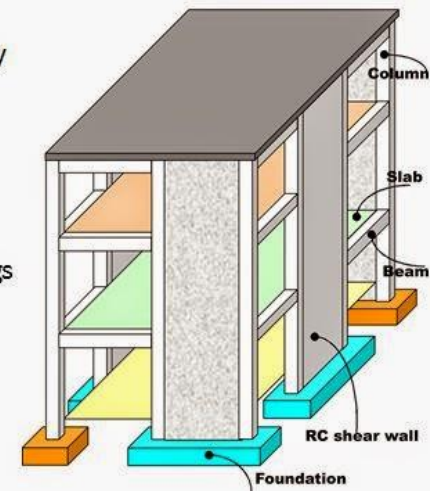


US-APWR Design Control Doc.

Project Scope

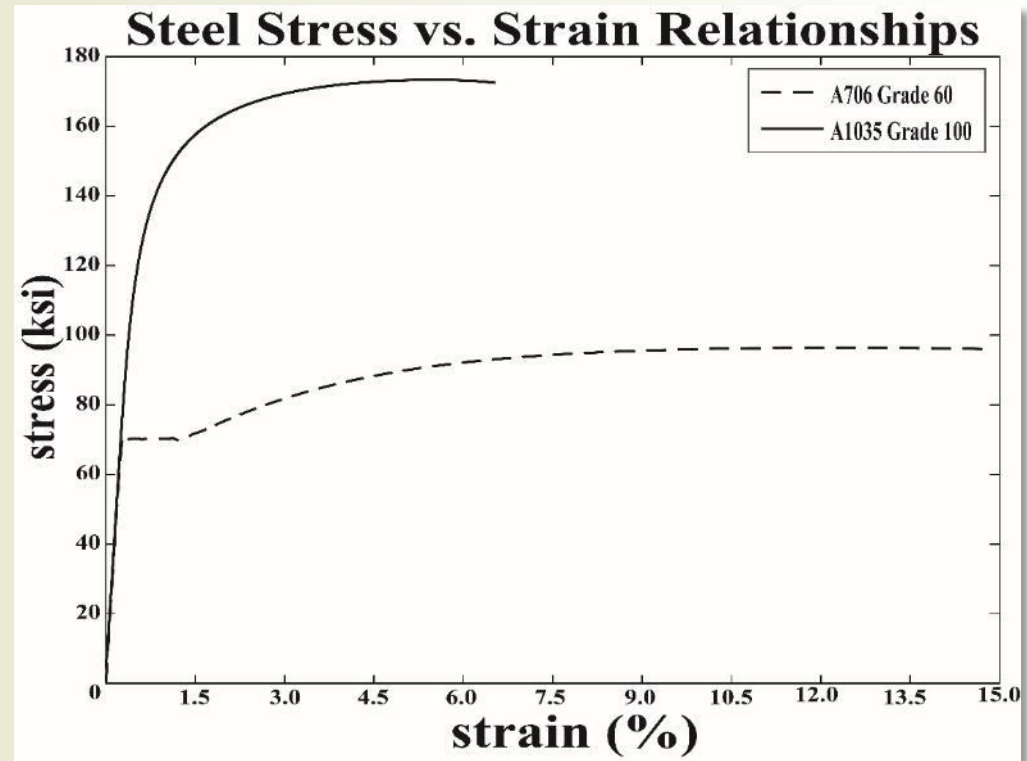
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- Aim to reduce complexities in rebar to improve construction quality and ease of inspection

RC shear walls carry earthquake loads down to the foundation. They provide large strength and stiffness to buildings in the direction of their orientation.



High-Strength Materials

- High-strength rebar (up to Grade 120) with high-strength concrete (up to 20,000 psi compressive strength)
- ACI 349 limits headed bars and shear reinforcement to Grade 60
- Concrete strength of 5,000 psi typical in current practice



Potential Benefits

**Most Congested
(current)**

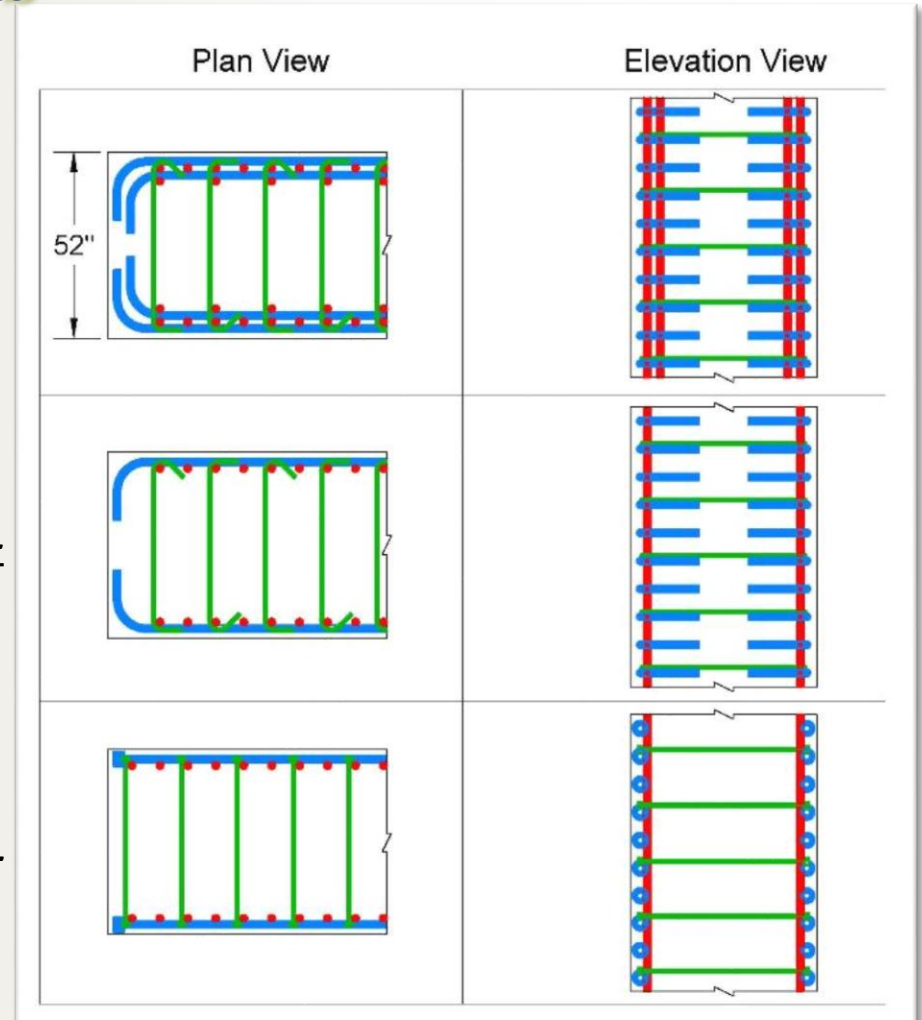


*Multiple layers
of hooked
Grade 60 bars*

*Fewer layers
of hooked high-
strength bars*

**Least Congested
(envisioned)**

*Fewer layers
of headed high-
strength bars*

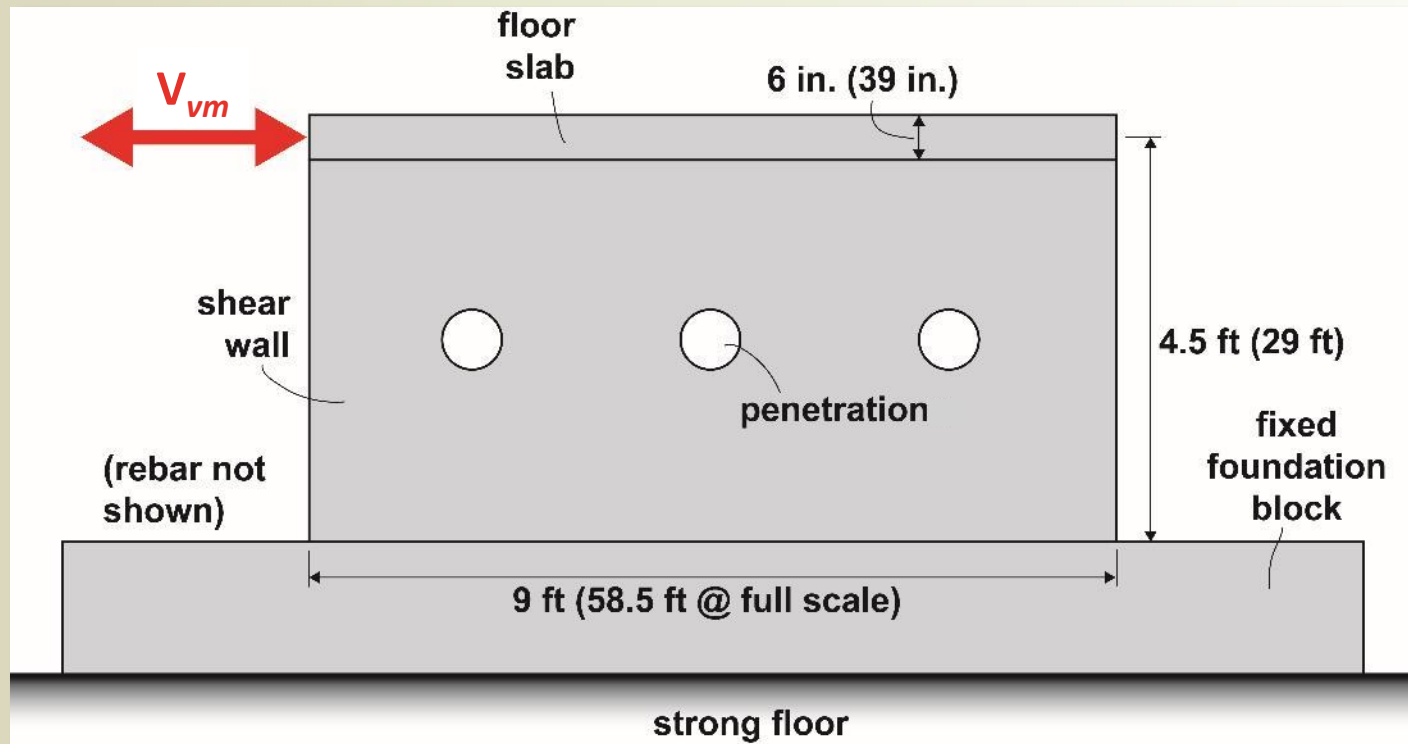


Outline

1. Numerical Modeling
2. Limit-Benefit Analysis
3. Cost-Benefit Analysis
4. Experimental Testing

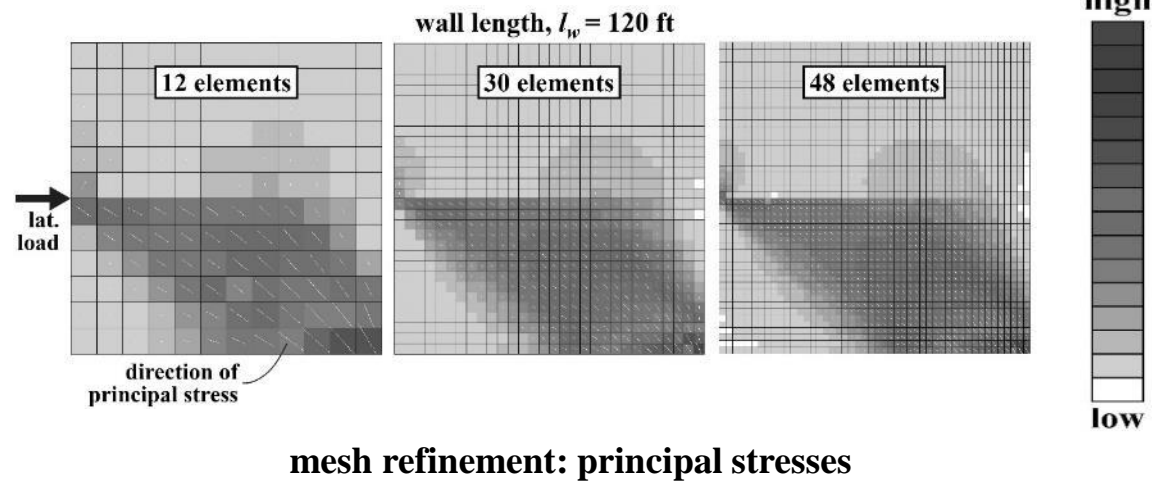
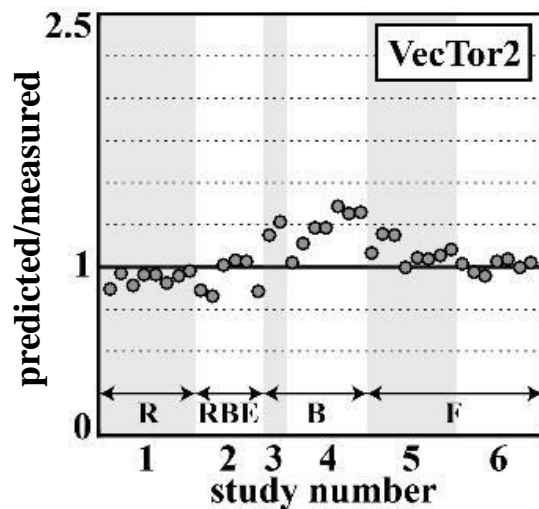
1. Modeling Approach

- Evaluated methods for predicting peak lateral strength (V_{vm}) of stocky shear walls:
 - 1) Closed-form Design Methods
 - 2) Finite Element Modeling Predictions



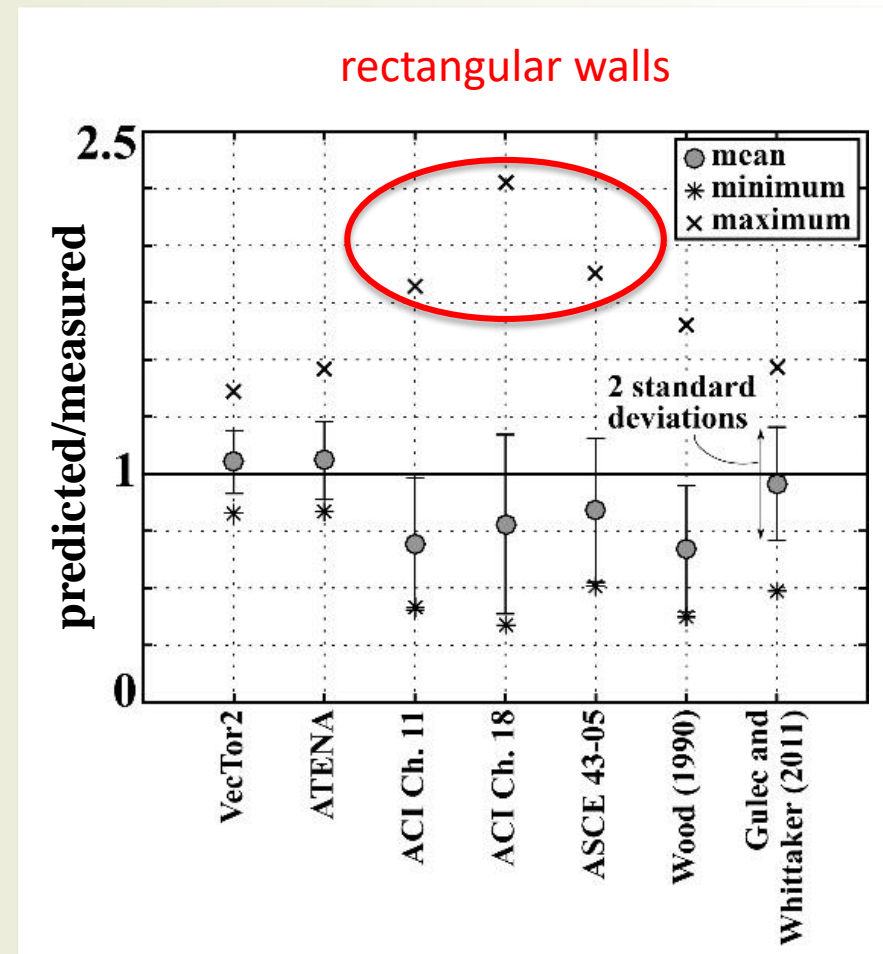
1. VecTor2 Finite Element Model

- Reliably captures the peak strength for rectangular walls with a wide range of material properties and base moment-to-shear ratios



1. Comparison of Predictions

- Design equations should be revisited, although mean predictions are conservative, there are unconservative outliers for typical nuclear wall geometries.
- VecTor2 and ATENA are reliable for predicting peak strength; ABAQUS will also be used.



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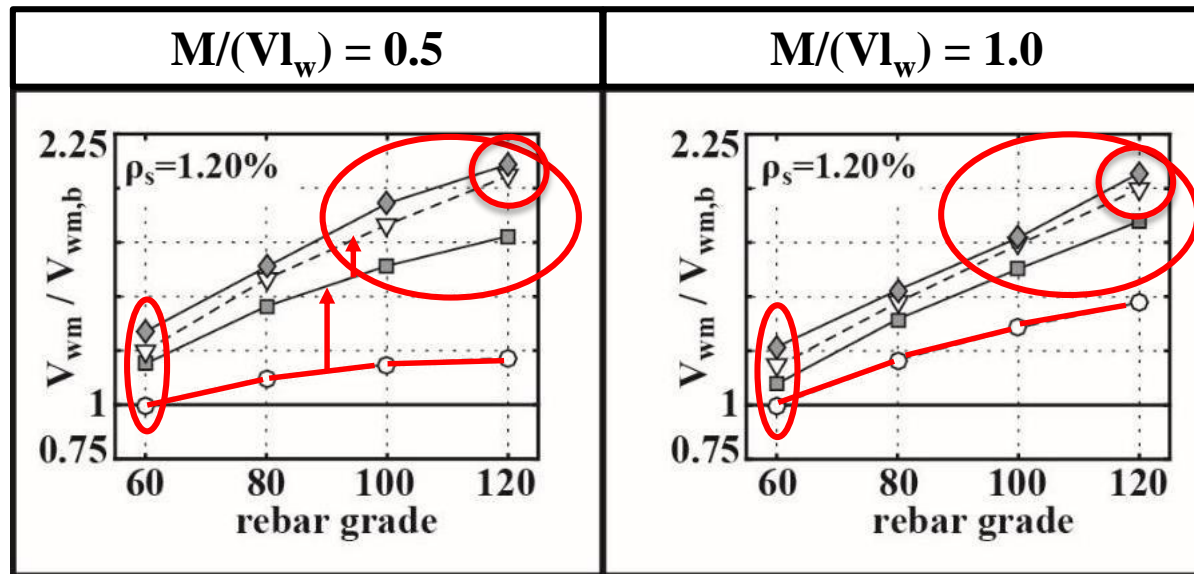
2. Limit-Benefit Analysis

- Numerical limit-benefit study to establish effects of high-strength materials on peak lateral strength of stocky shear walls:
 - Parametric numerical investigation of 192 walls
 - Peak strength predicted via VecTor2 finite element model

| Parameter | Wall 1 | Wall 2 | Wall 3 |
|-----------------------------------|-------------------------|-------------------------|-------------------------|
| length, l_w (ft) | 20 | 60 | 120 |
| height, h_w (ft) | 40 | 120 | 120 |
| thickness, t_w (in.) | 15 | 45 | 45 |
| moment to shear ratio, $M/(Vl_w)$ | 0.5, 1.0 | 0.5, 1.0 | 0.5, 1.0 |
| concrete strength, f'_c (ksi) | 5, 10, 15, 20 | 5, 10, 15, 20 | 5, 10, 15, 20 |
| rebar strength, f_y (ksi) | 60, 80, 100, 120 | 60, 80, 100, 120 | 60, 80, 100, 120 |
| reinforcement ratio, ρ_s (%) | 0.25, 0.50 | 0.60, 1.20 | 0.60, 1.20 |

2. Representative Limit-Benefit Results

Wall 2 (60 ft long, 120 ft tall, 45 in. thick):



—○— $f'_c = 5.00$ ksi

—□— $f'_c = 10.0$ ksi

—▽— $f'_c = 15.0$ ksi

—◇— $f'_c = 20.0$ ksi

V_{wm} = Predicted peak lateral strength

$V_{wm,b}$ = Predicted peak lateral strength of “benchmark” with normal strength materials

2. Limit-Benefit Summary

- Combination of high-strength rebar with high-strength concrete resulted in a higher-performing structure than with either high-strength material on its own
- Higher-strength concrete contributed more effectively for lower base moment to shear ratio walls; rebar yield strength becomes more significant as base moment to shear ratio increases
- Significant benefits are seen by using concrete compressive strength of 10 ksi, with diminishing returns for higher strengths
- Greatest benefits of high-strength materials for walls with large rebar amounts

Outline

1. Numerical Modeling
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3. Cost-Benefit Analysis

- Numerical cost-benefit study of economic effectiveness of high-strength materials for stocky shear walls:
 - Parametric numerical investigation of 2304 walls

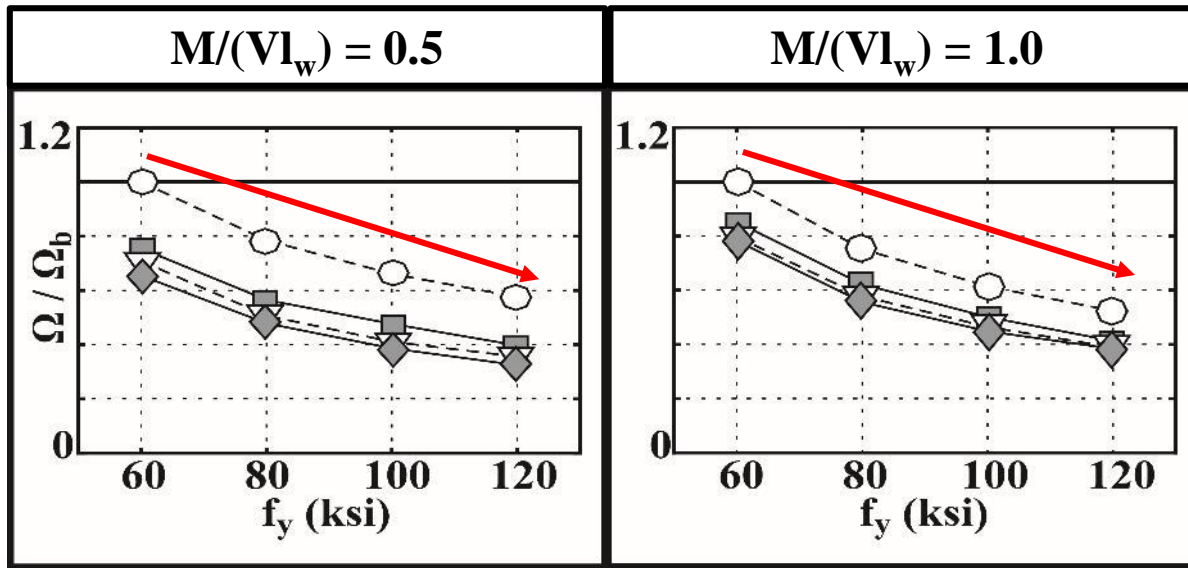
| Parameter | Wall 1 | Wall 2 | Wall 3 |
|---|--------------------------|--------------------------|--------------------------|
| length, l_w (ft) | 20 | 60 | 120 |
| height, h_w (ft) | 40 | 120 | 120 |
| thickness, t_w (in.) | 10, 15 , 20 | 30, 45 , 60 | 30, 45 , 60 |
| moment to shear ratio, $M/(Vl_w)$ | 0.5 , 1.0 | 0.5 , 1.0 | 0.5 , 1.0 |
| concrete strength, f'_c (ksi) | 5 , 10, 15, 20 | 5 , 10, 15, 20 | 5 , 10, 15, 20 |
| rebar strength, f_y (ksi) | 60 , 80, 100, 120 | 60 , 80, 100, 120 | 60 , 80, 100, 120 |
| reinforcement ratio, ρ_l (%) | low to high | low to high | low to high |
| ratio of reinforcement, ρ_t/ρ_l | 0.80, 1.00 | 0.80, 1.00 | 0.80, 1.00 |

3. Cost-Benefit Metrics

- Rebar weight factor (Ω) captures the total weight of rebar (w_r) normalized by peak strength (V_{wm}):
$$\Omega = \frac{w_r}{V_{wm}}$$
- Construction cost metric (Γ) includes rebar material cost, rebar labor cost, and concrete material cost (C_w), normalized by peak strength (V_{wm}):
$$\Gamma = \frac{C_w}{V_{wm}}$$
- Both metrics are then normalized by “benchmark” metrics (e.g. Ω_b , Γ_b) for walls with normal-strength materials

3. Rebar Weight Factor Results

Wall 2 (60 ft long, 120 ft tall, 45 in. thick), $\rho_l = \text{very high}$:



--○-- $f'_c = 5.00$ ksi

--□-- $f'_c = 10.0$ ksi

--▽-- $f'_c = 15.0$ ksi

--◇-- $f'_c = 20.0$ ksi

$$\Omega = \frac{w_r}{V_{wm}}$$

Ω = Rebar weight factor

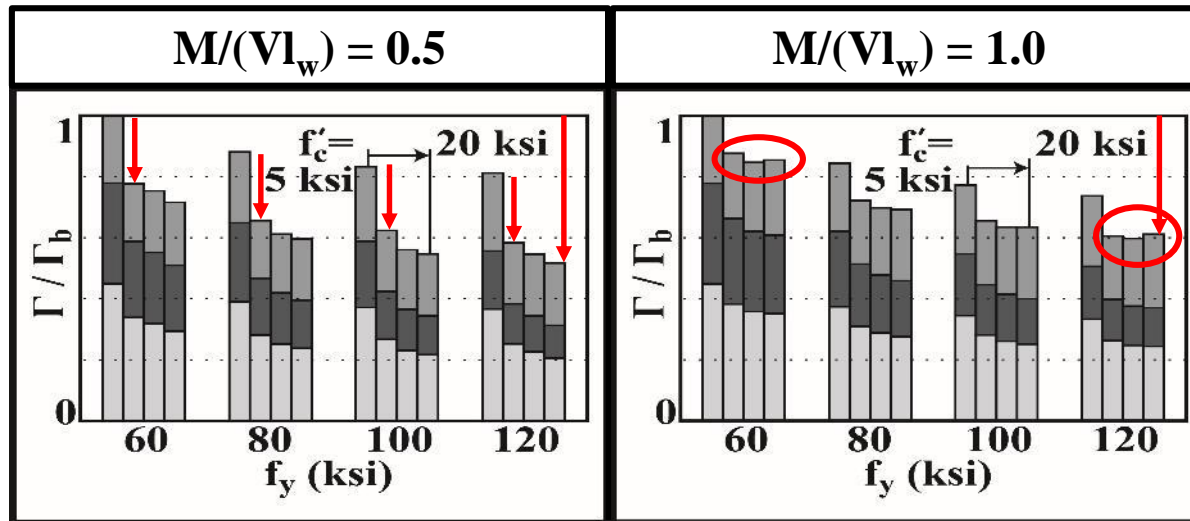
Ω_b = Rebar weight factor of “benchmark” with normal-strength materials

w_r = Total weight of longitudinal and transverse rebar

V_{wm} = Predicted peak lateral strength

3. Construction Cost Metric Results

Wall 2 (60 ft long, 120 ft tall, 45 in. thick), $\rho_l = \text{very high}$:



rebar (material)
 rebar (labor)
 concrete (material)

$$\Gamma = \frac{C_w}{V_{wm}}$$

Γ = Construction cost metric

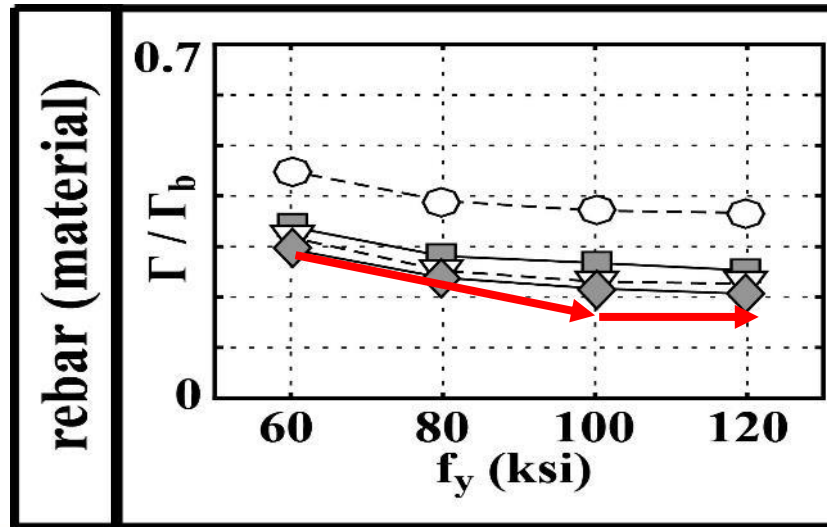
Γ_b = Construction cost metric of “benchmark” with normal-strength materials

C_w = Total cost of rebar material, rebar labor, and concrete material

V_{wm} = Predicted peak lateral strength

3. Rebar Cost Results

Wall 2 (60 ft long, 120 ft tall, 45 in. thick) with $M/(Vl_w)=0.5$, $\rho_l = \text{very high}$:



--○-- $f'_c=5.00$ ksi

--□-- $f'_c=10.0$ ksi

--▽-- $f'_c=15.0$ ksi

--◇-- $f'_c=20.0$ ksi

$$\Gamma = \frac{C_w}{V_{wm}}$$

Γ = Construction cost metric

Γ_b = Construction cost metric of “benchmark” with normal-strength materials

C_w = Total cost of rebar material, rebar labor, and concrete material

V_{wm} = Predicted peak lateral strength

3. Cost-Benefit Summary

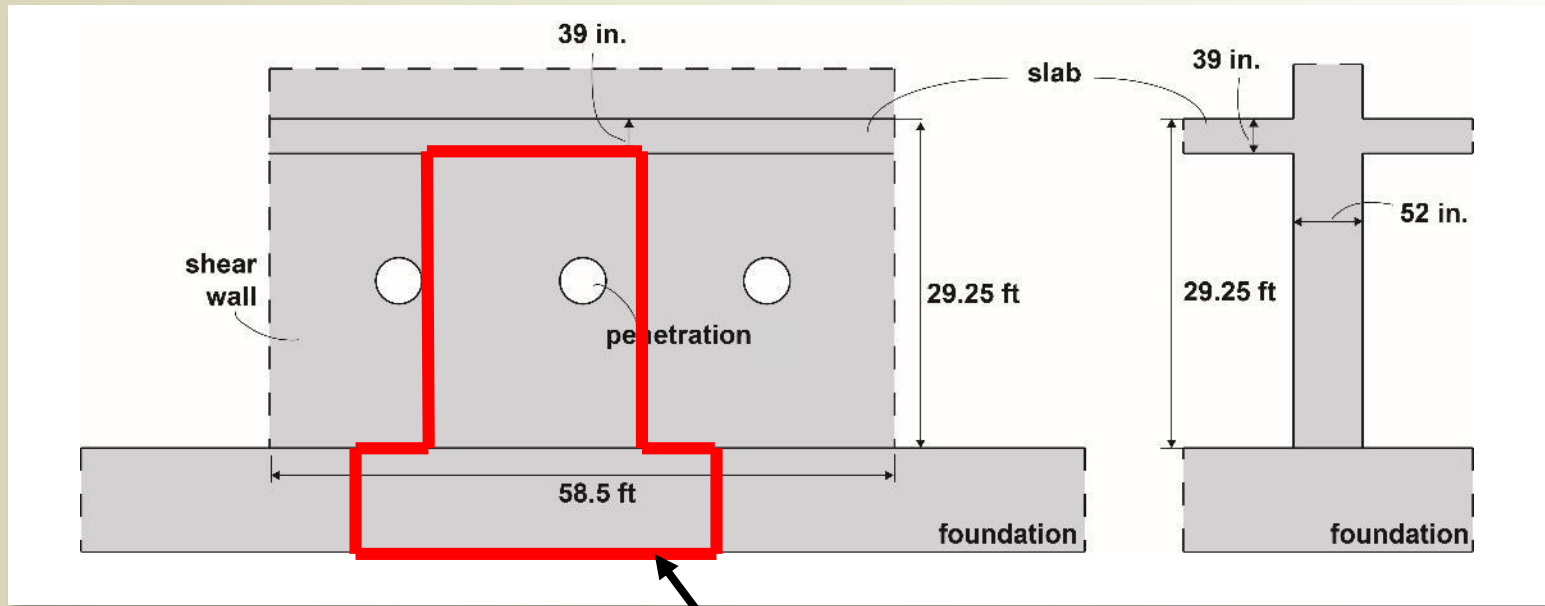
- For all walls, increasing the material strength of both concrete and rebar resulted in lower Rebar Weight Factors
- Combination of high-strength rebar with high-strength concrete resulted in greatest economic benefits
- A concrete compressive strength of 10 ksi showed the largest incremental reduction in construction cost; higher concrete strengths can increase normalized cost metric
- Rebar grades greater than 100 can lead to negligible economic benefits due to the increased unit cost

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4. Experimental Testing

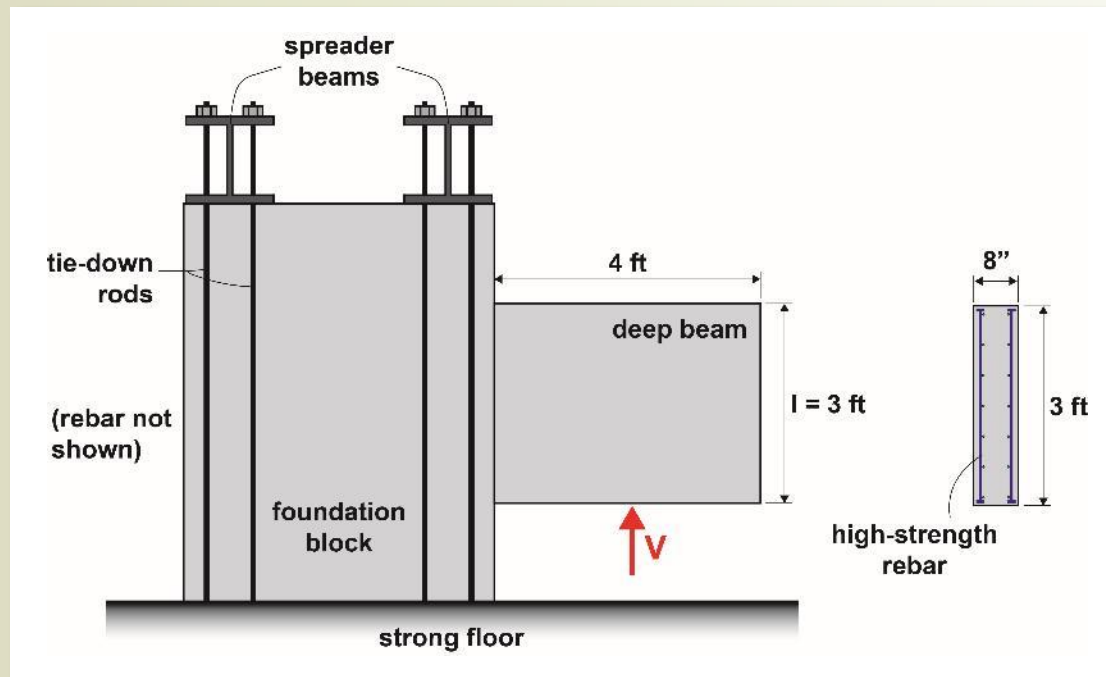
- “Generic wall” dimensions determined using publicly-available design control documents



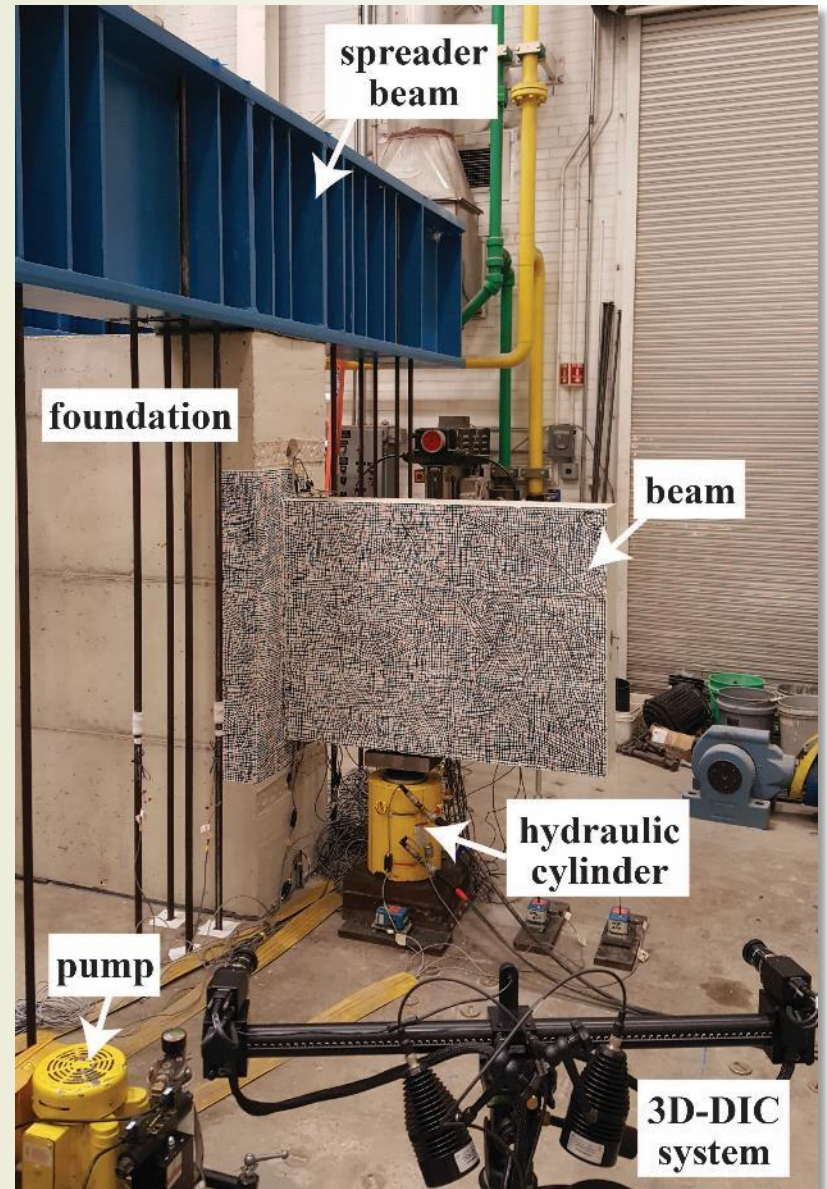
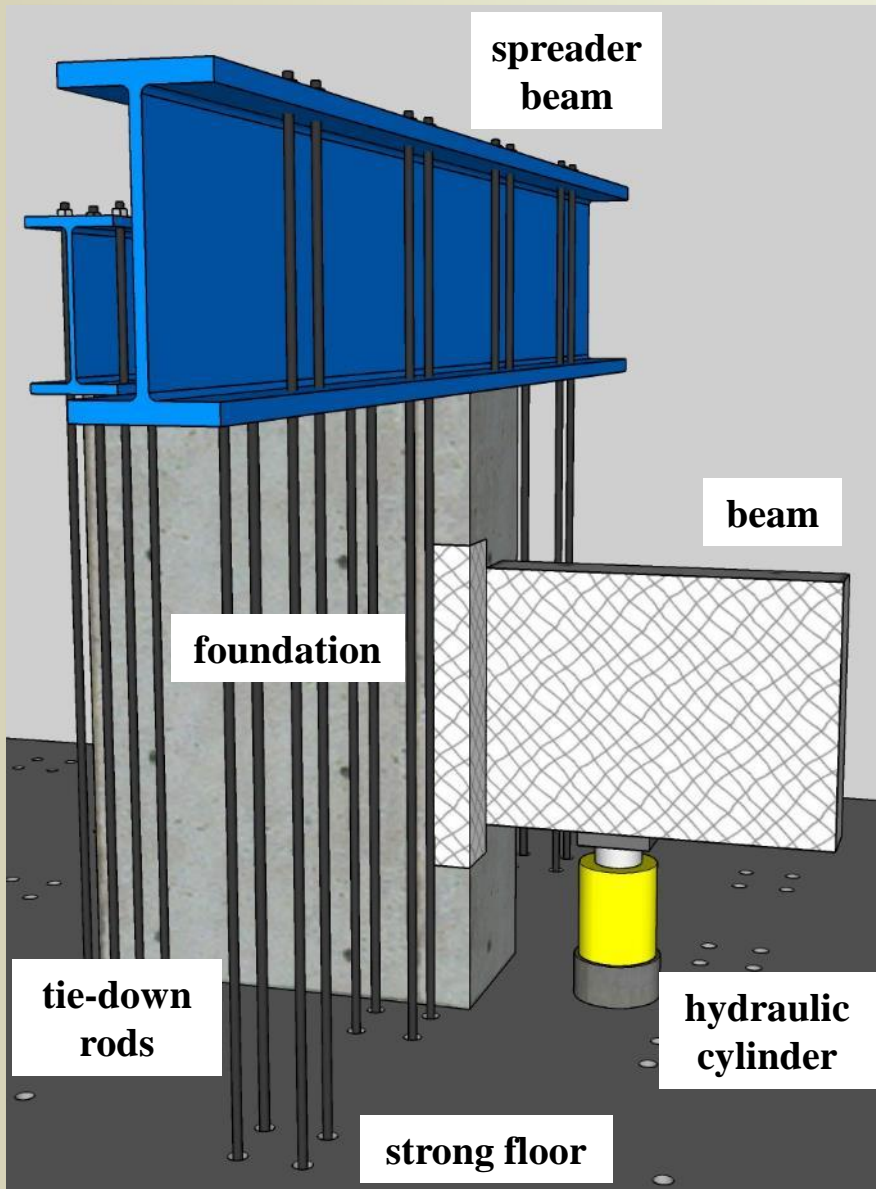
**representative slice of generic wall
for deep beam tests (@ 1:6.5 scale)**

4. Experimental Testing

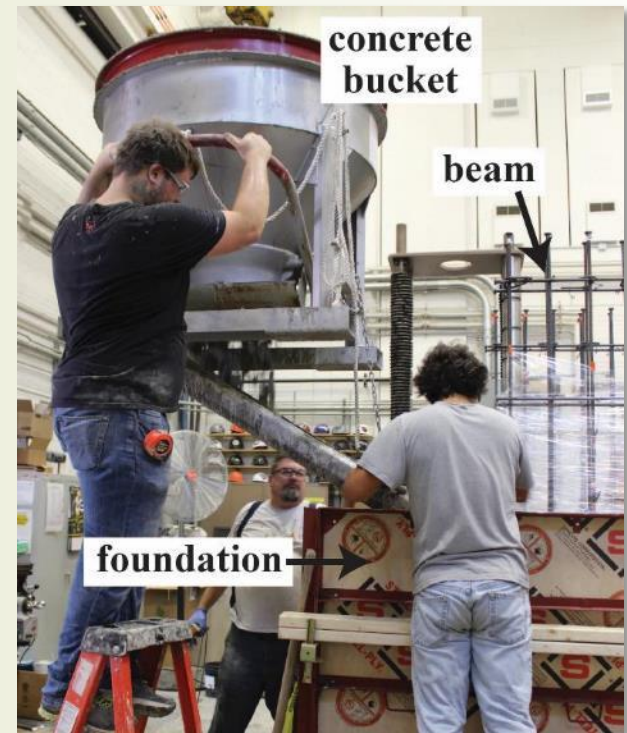
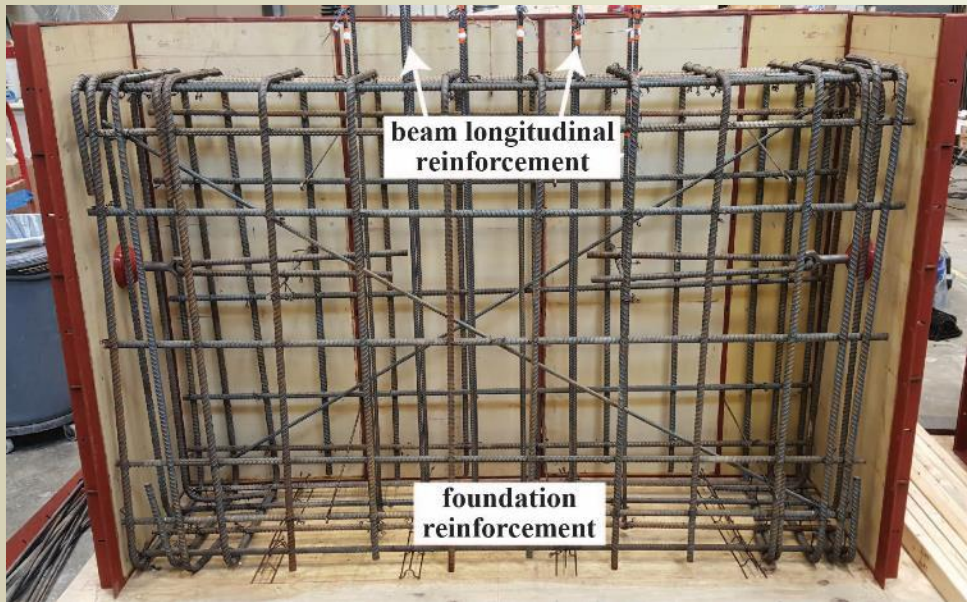
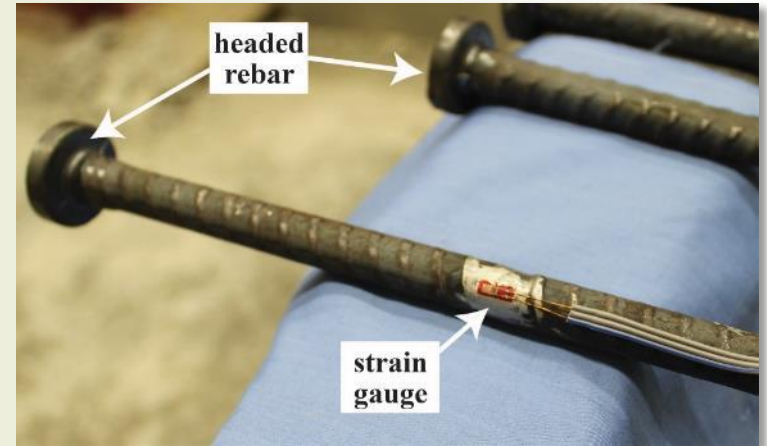
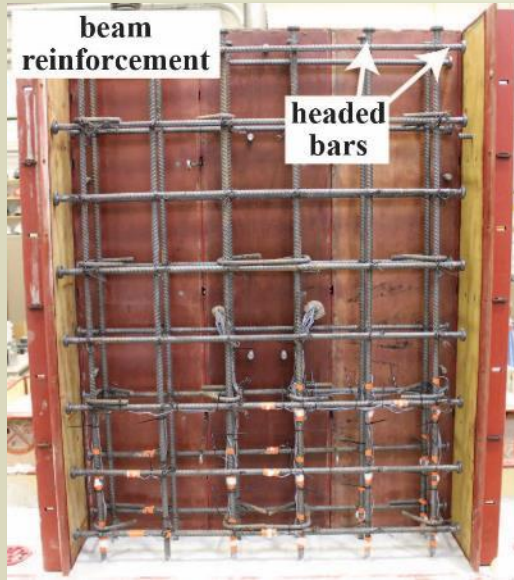
- “Generic wall” dimensions determined using publicly-available design control documents



4. Test Setup



4. Specimen Construction



4. Test Parameters

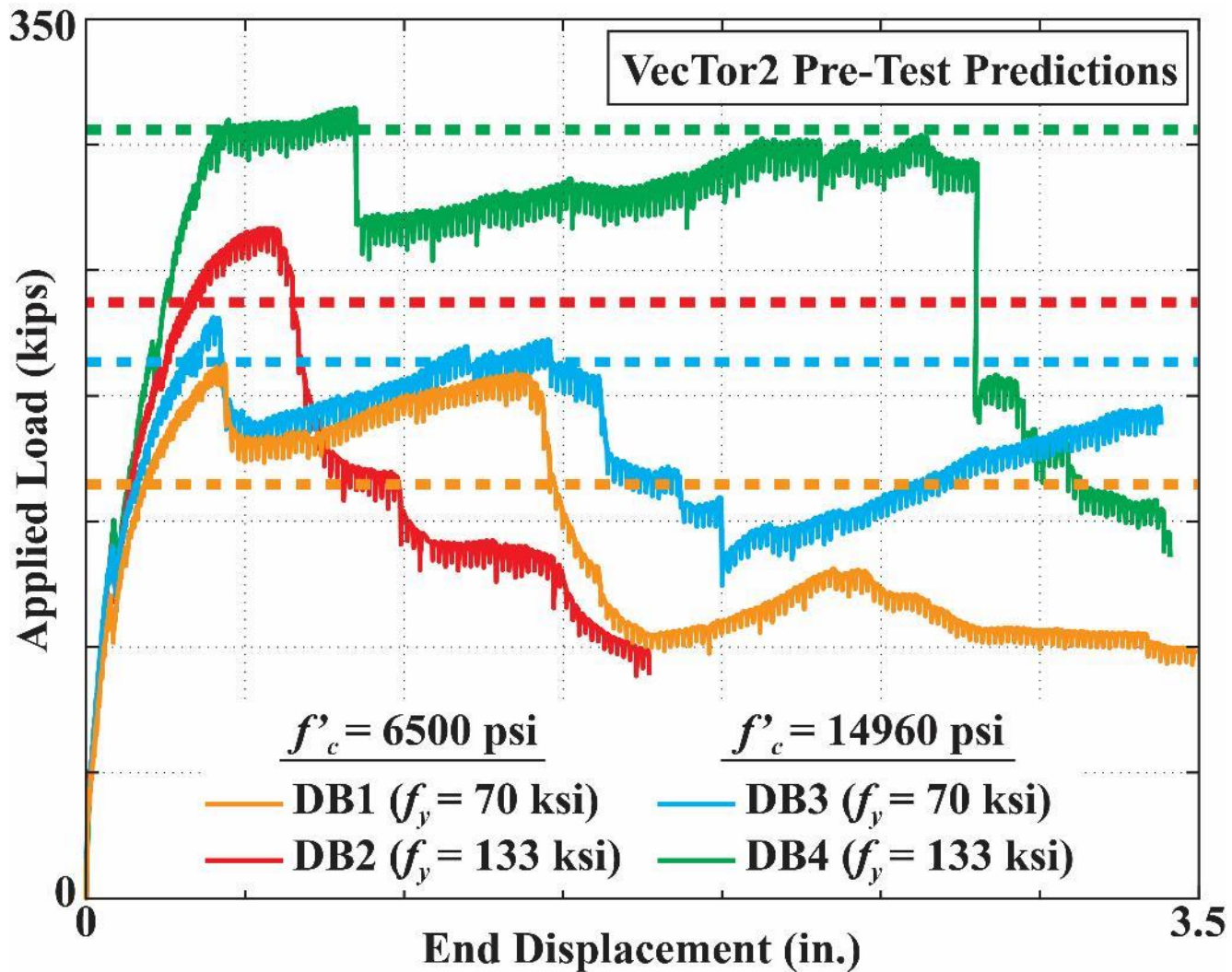
| Specimen | f'_c (psi) | f_y (ksi) | ρ_s (%) | $M/(Vl_w)$ |
|----------|--------------|-------------|--------------|------------|
| DB1 | 6500 | 70 | 0.833 | 0.5 |
| DB2 | 6500 | 133 | 0.833 | 0.5 |
| DB3 | 14960 | 70 | 0.833 | 0.5 |
| DB4 | 14960 | 133 | 0.833 | 0.5 |

Definitions: f'_c – concrete 28 day compressive strength

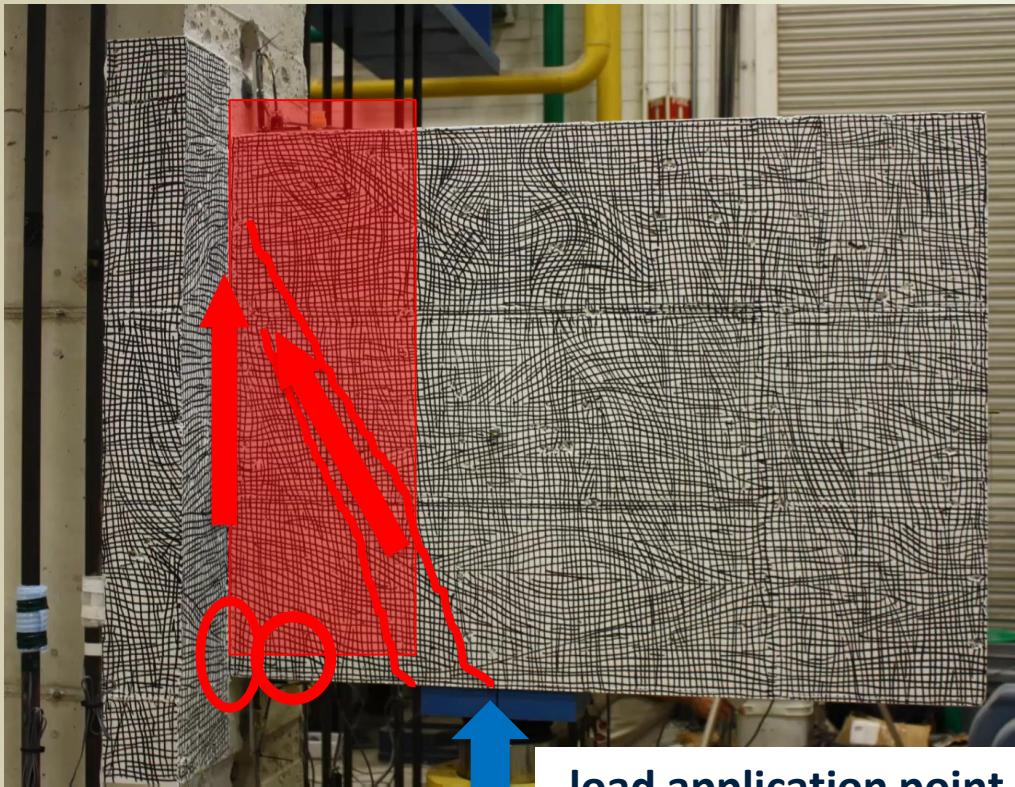
f_y – rebar yield strength, determined by tensile tests and 0.2% offset method

ρ_s – reinforcement ratio, symmetric for longitudinal and transverse rebar

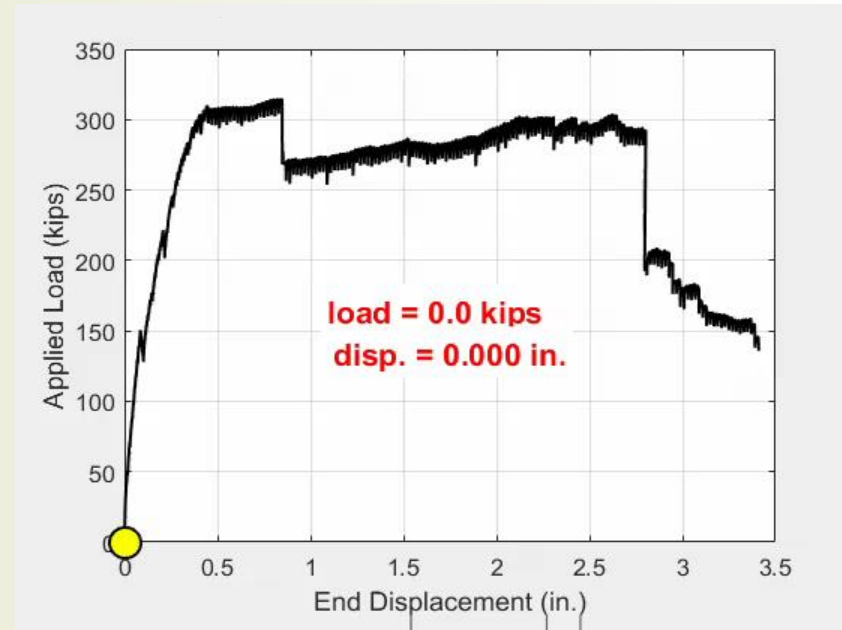
4. Specimen Response



4. DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)

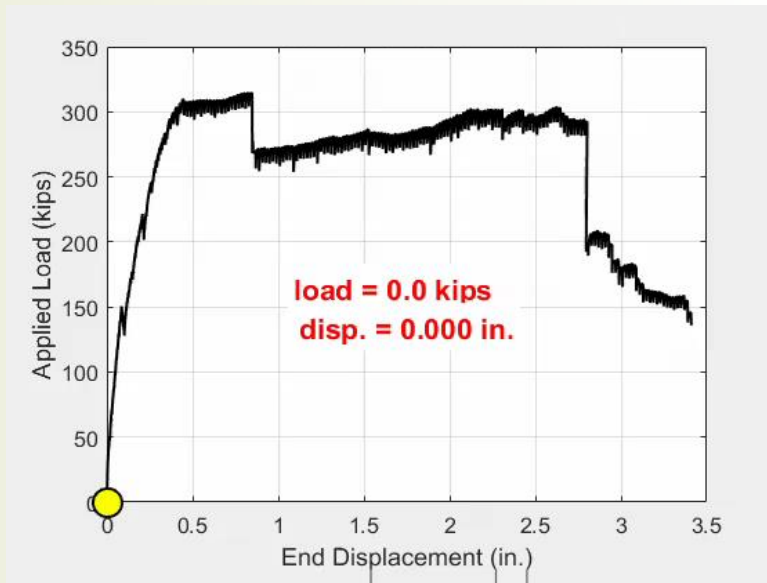
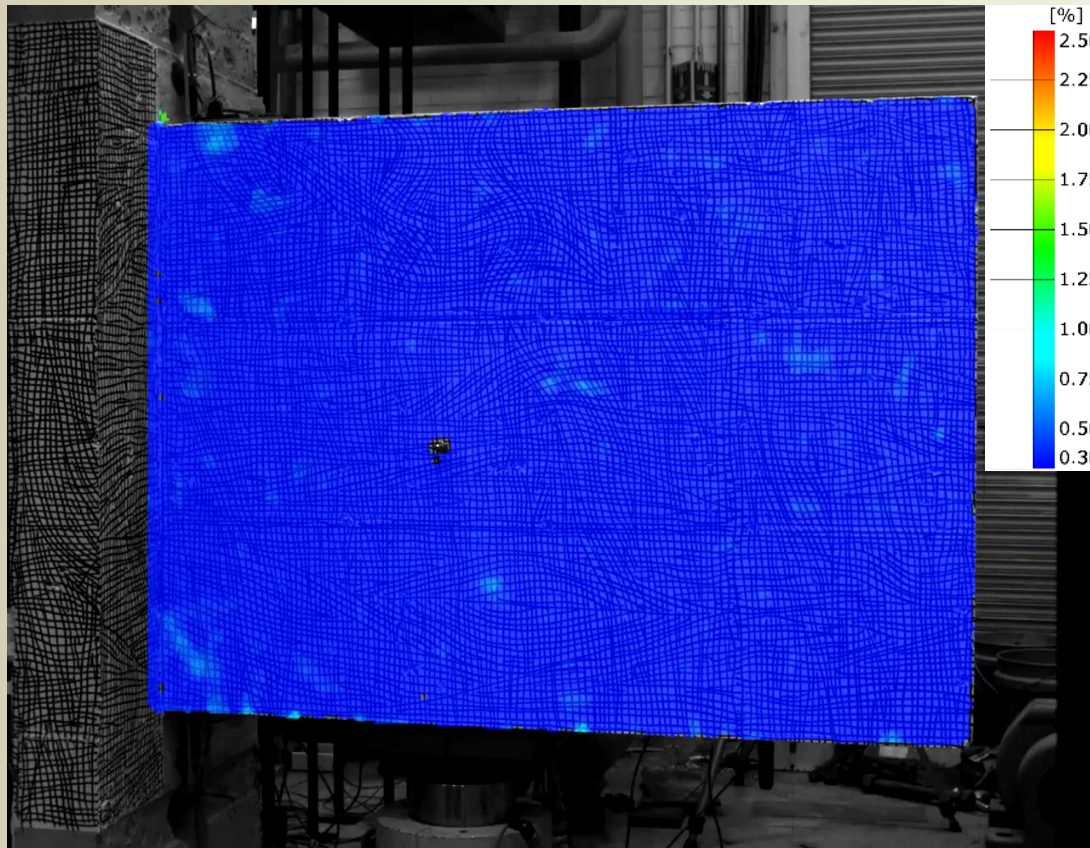


load application point



VIDEO, contact ykurama@nd.edu or athrall@nd.edu for more information

4. DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



VIDEO, contact ykurama@nd.edu or athrall@nd.edu for more information

4. Summary of Tests

- Most significant strength increase and most ductile failure for deep beams was when high-strength materials were used together (DB4)
- Isolated increase in rebar yield strength (DB2) resulted in higher increase in deep beam strength than isolated increase of concrete compressive strength (DB3)
- Pre-test analyses provided reasonable and conservative predictions for all specimens

Conclusions

- High-strength steel more effective when combined with high-strength concrete
 - Numerically demonstrated (economics and peak strength)
 - Measured experimentally
- Greatest benefit for walls with low base moment to shear ratios and large reinforcement amounts; typical of nuclear concrete shear walls
- Largest economic and structural benefits when using Grade 100 rebar together with 10 ksi compressive strength concrete

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- Scott Sanborn (Sandia National Laboratories)
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 - Dayton Superior Corp.
 - HRC, Inc.
 - Sika Corp. U.S.



Questions?



<http://phsrc-nuclearwalls.nd.edu>

